#### APPENDIX 7.2 2006 SITE INVESTIGATION

Proposed Development at Portmarnock, Co. Dublin

Report No. 06-693

**Client: Ballymore Residential Ltd** 

Engineer: JB Barry & Partners Ltd

## SITE INVESTIGATION

# Proposed Development at Portmarnock, Co. Dublin

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#### Methods of describing soils and rocks

Soil and rock descriptions are based on the guidance in Section 6 of BS 5930: 1999, *The Code of Practice for Site Investigation*, with the following exceptions:

- 1. Where the strength of clay is based on field assessment without the availability of laboratory or in-situ test results the following terms are used, where applicable:
- soft to firm: clay with undrained shear strength close to the BS5930 boundary (40kPa) between soft and firm soil.
- firm to stiff: clay with undrained shear strength close to the BS5930 boundary (75kPa) between firm and stiff soil.
- 2. The relative density of coarse-grained soils, described in trial pit logs, is based on field observations including stability of pit sides and the ease/difficulty of excavation. The description is for indicative purposes only: as required by BS 5930, the relative density should only be determined by use of insitu tests, including standard penetration tests.

Abbreviations	used on exploratory hole logs
U	Nominal 100mm diameter undisturbed open tube sample
Р	Nominal 100mm diameter undisturbed piston sample
В	Bulk disturbed sample
D/J	Small disturbed sample
W	Water sample
ES	Soil sample for environmental testing
SPT	Standard penetration test using a split spoon sampler (small disturbed sample obtained)
CPT	Standard penetration test using 60 degree solid cone
X,X/X,X,X,X	Blows per increment during the standard penetration test. The initial two values relate to the seating drive (150mm) and the remaining four to the 75mm increments of the test length.
N=X	SPT blow count 'N' given by the summation of the blows 'X' required to drive the full test length (300mm)
X*/Y	Incomplete standard penetration test where the seating drive could not be completed. The blows 'X' represent the total blows for the given length of seating drive 'Y' (mm)
X/Z	Incomplete standard penetration test where the seating drive was achieved but the full test length was not. The blows 'X' represent the total blows for the given test length 'Z' (mm)
V	Shear vane test (borehole) Hand vane test (trial pit) Shear strength stated in kPa
VR	V: undisturbed vane shear strength VR: remoulded vane shear strength
dd/mm/yy: 1.0 dd/mm/yy: dry	Date & water level at the borehole depth at the end of shift and the start of the following shift
	s relating to rock core - reference Clause 44.4.4 of BS 5930: 1999
TCR (%)	Total Core Recovery: Ratio of rock/soil core recovered (both solid and non-intact) to the total length of core run.
SCR (%)	Solid Core Recovery: Ratio of <i>solid core</i> to the total length of core run. <i>Solid core</i> has a full diameter, uninterrupted by natural discontinuities, but not necessarily a full circumference and is measured along the core axis between natural fractures.
RQD (%)	Rock Quality Designation: Ratio of total length of <i>solid core</i> pieces greater than 100mm to the total length of core run.
FI	Fracture Index: Number of natural discontinuities per metre over an indicated length of core of similar intensity of fracturing.
NI	Non Intact: Used where the rock material was recovered fragmented, for example as fine to coarse gravel size particles.

## SITE INVESTIGATION

## Proposed Development at Portmarnock, Co. Dublin

#### 1 AUTHORITY

On the instructions of the JB Barry & Partners Ltd, acting on behalf of the Client Ballyore Residential Ltd. a ground investigation was undertaken at the above site to establish the subsoil conditions with regard to the proposed development (Appendix A).

#### 2 SCOPE

The extent of the investigation was as directed and designed by the Engineer and included the sinking of cable percussion boreholes, the installation of groundwater monitoring wells, trial pits, soak away tests, sampling, in-situ and laboratory testing, and the preparation of a report on the findings.

#### 3 DESCRIPTION OF SITE

The site is located south of Portmarnock, it is bounded to the west by the Belfast – Dublin railway and to the south and east by the R123 and R106 respectively. The majority of the site is within private agricultural land which is divided into fields with small hedgerows along some boundaries.

A site location plan is presented in Appendix A.

#### 4 FIELDWORK

The fieldwork undertaken for the investigation was between 20 September 2006 and 13 October 2006 under the supervision of an Engineer from Glover Site Investigations Ltd.

The number, location and depths of exploratory holes were as directed by the Engineer.

#### 4.1 Boreholes by cable percussion drilling methods

Six boreholes were sunk by means of a Dando 2000 drilling rig using shell and auger techniques. The boreholes were drilled 200mm diameter using temporary casing and boring tools and extended to depths of 10.00m below existing ground level.

Disturbed (small jar and bulk bag) samples were taken from the strata encountered.

Any water strikes encountered during boring were recorded along with any changes in their levels as the borehole proceeded.

The borehole locations are shown on the plan in Appendix A and its log is provided in Appendix B.

#### 4.1.1 Standard Penetration Tests

Standard Penetration Tests, using either a split barrel sampler (SPT) or solid 60' cone (CPT) to enable measurement of the penetration resistance (N) to be determined under dynamic loading. When the number of blows for the seating drive exceeds 25 blows the distance driven is recorded and the test drive started. If the test drive exceeds 50 blows (increased to 100 blows in soft rock) then the distance driven is recorded and the test drives driven is recorded and the test drives driven is recorded.

#### 4.1.2 Groundwater Monitoring Installations

Installations in selected boreholes (BH's 1, 2, 3, 4, 5 & 6) included 50mm standpipes. Details of individual installations are presented on the instrumentation data sheet that accompanies the relevant borehole logs (Appendix B).

Standpipe depths range from 9.60m - 10.00m depth below existing ground level. Gas valves where fitted to standpipe installations.

#### 4.2 Trial pits

Six trial pits were excavated using a 3 tonne tracked excavator fitted with a 600mm wide toothed bucket. The pits extended to depths of 3.00m

Disturbed (bulk bag) and environmental samples were taken of the strata encountered.

Any groundwater entries to the pits were recorded along with any rise in groundwater during excavation. The stability of the pit walls was noted.

The trial pit logs are provided in Appendix C.

#### 4.3 Percolation tests

Three percolation tests were conducted in a further three trial pits at locations P1, P2 & P3. The tests were conducted in accordance with CIRIA Report 156 (1996), *Infiltration drainage – Manual of good practice*.

The percolation results of the tests are provided in Appendix D. However, the static water level during the tests precluded the calculation of the infiltration coefficient of the soil.

#### 4.4 California Bearing Ratio Tests

California Bearing Ratio (CBR) tests where completed at trial pit locations which are at or close to grade using a Dynamic Cone Penetrometer. Results of in situ testing are given in Appendix E.

#### 4.5 Exploratory Hole Position and Elevation

All exploratory hole excavations were surveyed by means of a Trimble Global Positioning System (GPS). An ordnance survey GPS base station was used to give us Irish grid co-ordinates and elevation, from this information we are able to survey the site with an accuracy of +/- 20mm. Exploratory hole co-ordinates and elevations are presented on each exploratory hole log.

#### 5 LABORATORY WORK

Upon their receipt in the laboratory, all disturbed samples were carefully examined and accurately described and their descriptions incorporated into the preliminary borehole logs. The logs were revised, where necessary, based on the results of the laboratory tests. Laboratory testing, conducted as scheduled by the Engineer, comprised the following:

- Classification tests: moisture content, particle size distribution and Atterberg Limits,
- Soil/groundwater chemistry tests: pH and sulphate content

The test results are tabulated in Appendix F (parts i-iii) unless noted otherwise; tests were conducted in accordance with BS 1377:1990, *Methods of test for soils for civil engineering purposes*. *Parts 1 to 9*.

#### 6 PROPOSED CONSTRUCTION

It is proposed to construct a housing development with associated access roads and car parking.

#### 6.1 Ground types

Examination of the exploratory hole logs reveals that the general ground conditions can be summarised as follows;

Exploratory Hole Location	Topsoil (m thickness)	Made Ground (m thickness)	Peat (m thickness)	Peaty Clays (m thickness)	Sands and Gravels (m thickness)	Glacial Clays (m thickness)	Water ingress m b.g.l
						>10.00	Dry
BH 1	0.40	-	20	-	-		
BH 2	0.30		-	-	-	>10.00	7.40
BH 3	0.40		-	-	(B)	>10.00	Dry
BH 4	0.40	-	-	-	-	>10.00	Dry
BH 5	0.40	-	-	-	-	>10.00	Dry
BH 6	0.40	-	-	-	-	>10.00	Dry
TP 1	0.25	-	-	-	-	>2.50	Dry
TP 2	0.25	-			-	>3.00	Dry
TP 3	0.25	1	-	-	-	>2.60	Dry
TP 4	0.20	-	-	-	-	>2.60	Dry
TP 5	0.25	-	-	-	-	>2.60	Dry
TP 6	0.25	-	-	-	-	>2.60	Dry

(m b.g.l - metres below ground level)

No rock was encountered.

#### 6.2 Groundwater

Groundwater was encountered only encountered in borehole 2 at 7.40m which was under sub artesian pressure and rose to 7.00m below existing ground level.

All other exploratory hole were dry during excavation.

### 7 PRELIMINARY FOUNDATION DESIGN

#### 7.1 Proposed housing development

At the time of preparation of this report no detailed information was available with respect to foundation loading or anticipated final ground levels and structures to be constructed.

In this report it has been assumed that all foundations in natural ground, if suitable, will as a minimum be placed 0.80m below finished ground level to guard against the effects of frost action.

Strip or pad foundations placed at 1.00m depth within the firm to stiff glacial soils may be suitable with an allowable bearing capacity of 120kN/m<sup>2</sup>. On removal of topsoil floating ground floor slabs placed on the glacial soils would be feasible.

Where ground levels are raised by filling, floating slabs might be subject to unpredictable settlement and the use of suspended floors in these areas should be considered.

If cut and fill techniques are planned for the site, consideration could be given to compaction trials to ensure that a maximum amount of material is re-used in fill areas. Fill material should be placed in layers in accordance with current earthworks standards and in situ measurements of dry density should be carried out on a regular basis to ensure that the required compaction is being achieved.

#### 7.2 Proposed Pavements

No information was supplied during the preparation of this report with regard to final ground levels of the proposed access roads. However, it is assumed that the levels of the proposed roads will be similar to existing.

Topsoil and vegetation shall be thoroughly stripped away in areas of filling. Following the topsoil strip, any localised areas of soft ground shall be excavated and backfilled with granular fill.

The following assess the sub-grade along the route, applying the recommendations of the Department of Transport's HD 25/94. This document relates soil properties to California Bearing Ratio (CBR) and the design CBR to the required capping layer and sub-base thickness.

Where the sub-grade is on existing in-situ soils, the soils will comprise glacial clays: their

properties, at or near sub-grade level vary.

In-situ California Bearing Ratio (CBR) tests where completed at six locations and results presented give a range of CBR values of between 2%- 12% at 250mm below existing ground level with an average of 6%. A CBR of 4% requires a 300mm capping and a 150mm thick subbase or 280mm sub-base (Figure 3.1 of HD 25/94).

However we would recommend that in-situ CBR's are undertaken when the sub-grade is exposed in order to assure that the capping/sub-base thickness is correct.

The use of geotextiles may enable a reduction in construction thickness. The advice of specialist contractors should be sought on this matter.

A narrow filter drain or fin drain shall be provided on the low side(s) of pavement layers to collect and dispose of water seeping into the pavement. The pipe for such a system shall be set lower than the sub-grade top surface to enable the system to act as a sump for percolation along the interface between the capping and sub-grade.

All materials used within 450mm of the finished road surface level shall be non frost-susceptible, as required by the Specification for Highway Works (Series 700) and tested according to BS 812 : Part 124 : (1989).

#### 8 SOIL AGGRESSIVENESS

Chemical tests (pH and total SO<sub>4</sub> contents) on soil samples (Appendix F part i) show a range between 24mg/l - 78mg/l - reference Table C2 of BRE Special Digest 1 (2005). The table shows a design Sulphate limit based on 2:1 water/soil extract, for Class DS-1 the limits are <500mg/l and class DS-2 between 500-1500mg/l. We would suggest that a design Sulphate class DS-1 can be adopted for the site with ACEC Class AC-1s.

#### 9 BRIEF GEOLOGY OF THE SITE

The Geological Survey of Ireland geological map of the area sheet 13, Meath Solid Edition, 1:100000 series shows the bedrock to be of the Carboniferous in age and consists of the Malahide Formation which is comprised of argillaceous bioclastic limestone, shale (Appendix G).

#### 10 REFERENCES

British Standards Institute (1999) BS 5930:1999, Code of practice for site investigations.

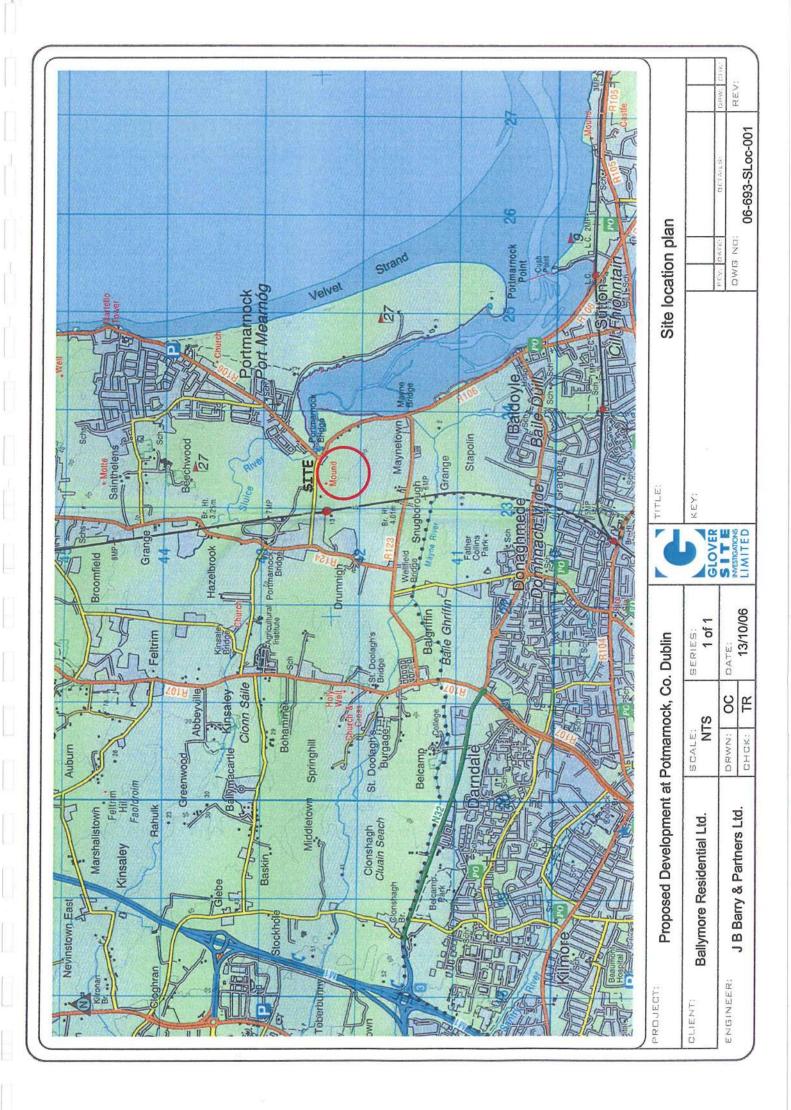
British Standards Institute (1990) BS 1377:1990, Methods of test for soils for civil engineering purposes. Parts 1 to 9.

Geological Survey of Ireland (1995) Geology of South Cork, Sheet 25, Scale 1:100,000

## APPENDIX A

**Site Location Plans** 

GLOVER SITE INVESTIGATIONS LIMITED





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# APPENDIX B

**Exploratory Borehole Logs** 

Glo	ver Sit	e In	ve	stigatio			Site Proposed Development at Portmarnock, Co. Dublin	Bore Num BH	ber
Boring Meth Cable Percu			Diamete Omm cas	<b>r</b> ed to 10.00m		Level (mOD) 14.16	Client Ballymore Residential Ltd	Job Num 06-	iber 693
		Locatio AS	n FLAN			/10/2006- /10/2006	Engineer J B Barry & Partners Ltd		/1
Depth (m)	Sample / Tests	Casing Depth (m)	Water Depth (m)	Field Records	Level (mOD)	Depth (m) (Thickness	Description	Leger	P. Water
0.00-0.50	B1					(0.40)	TOPSOIL		
0.50-0.95 0.50-0.95	CPT N=18 B2			2,2/4,4,5,5	13.76	0.40	Firm to stiff brown slightly sandy gravelly CLAY containing smooth sub-rounded cobbles. Gravel is sub-angular fine to coarse		
1.50-1.95 1.50-1.95	CPT N=23 B3			1,3/4,6,7,6	12.16	(1.60)	Stiff to yopy stiff black sandy gravelly CLAY containing		
2.50-2.95 2.50-2.95	CPT N=39 B4			4,7/8,9,11,11			Stiff to very stiff black sandy gravelly CLAY containing occasional smooth sub-rounded cobbles and boulders. Gravel is sub-angular fine to coarse		
3.50-3.95 3.50-3.95	CPT N=47 B5			3,7/11,12,11,13		the late to			
4.50-4.95 4.50-4.95	CPT N=48 B6			4,8/10,10,12,16		and a balantic last at			
6.00-6.45 6.00-6.45	CPT N=42 B7			3,6/9,11,11,11					<u></u>
7.50-7.80 7.50-7.95	CPT 50/150 B8			5,9/10,17,23		ուսեսու			
	G			12/10/2006:DRY 13/10/2006:5.80m	-	հոստես			
9.00-9.45 9.00-9.45	CPT N=37 B9			3,6/8,8,10,11		ուսեստե			
				13/10/2006:DRY	4.16				
Remarks Standpipe i Chiselling f	installed to 10.00m. rom 5.40m to 5.60m	for 0.75 h	iours. Wa	iter added from 0.50m			Scale (approx	) Lo By	ggeo
ornsening i	1011 0, 4011 (0 0.0011						1:50		R/HH
							Figure 06-	No. 693.B⊦	101

		r Si	te Ir	nvestigati	ons	Lto	k	Site Proposed	Develop	ment at P	ortmarno	ck, Co. D	ublin	N I	Borehole lumber BH01
Installatio WATER I	MONITOR	ING		I Diameter of Tube [A] = 50 ter of Filter Zone = 200 mm	mm			Client Ballymore	Residen	tial Ltd				N	lob lumber 06-693
			Location AS PL		Ground I	<b>.evel (m</b> 4.16	OD)	Engineer J B Barry	& Partnei	rs Ltd				S	Sheet 1/1
Water Vater	Instr (A)	Level (mOD)	Depth (m)	Description				G	roundwa	ter Strik	es Durin	g Drilling	1		
		13.96	0.20	Concrete Cement/Bentonite Grout	Date	Time	Depth Struck (m)	Casing Depth (m)	Inflov	w Rate	5 min	Read		20 min	Depti Seale (m)
		13.16	1.00	Gravel Filter											
0 0 0 0		12.66	1.50												
								Gr	oundwat	ter Obse	rvations	During [	) Drilling	1	
								Start of S	hift			I	End of SI	hift	
-0 -0					Date	Time	Dept Hole (m)	h Casing Depth (m)	Water Depth (m)	Water Level (mOD)	Time	Depth Hole (m)	Casing Depth (m)	Water Depth (m)	Wate Leve (mOl
					12/10/06 13/10/06	0800	8.00	8.00	5.80	8.36	1700	8.00 10.00		DRY DRY	
								Instr	ument G	roundwa	nter Obse	ervations			
					Inst.	[A] Type	:								
				Slotted Standpipe		Ins	trumer	nt [A]				Bom	arks		
					Date	Time	Dept (m)	h Level (mOD)				Ken			
		4.16	10.00												

Upright cover fitted.

oring Meth	nod	Casing	Diameter	stigatio	Ground	Level (mOD) 10.23	Client Ballymore Residential Ltd	Job Numl 06-6	
		Locatio AS	n PLAN			/10/2006- /10/2006	Engineer J B Barry & Partners Ltd	Shee 1/	
Depth (m)	Sample / Tests	Casing Depth (m)	Water Depth (m)	Field Records	Level (mOD)	Depth (m) (Thickness)	Description	Legen	d
.00-0.50	B1				9.93	(0.30) 0.30	TOPSOIL		
.50-0.95 .50-0.95	CPT N=25 B2			1,3/5,6,7,7	0.00	1.1.1.1.1.1.	Firm to stiff brown slightly sandy gravelly CLAY containing smooth sub-rounded cobbles. Gravel is sub-angular fine to coarse		
.50-1.95 .50-1.95	CPT N=23 B3			2,4/6,5,6,6					
.50-2.95 .50-2.95	CPT N=35 B4			3,5/7,9,8,11	8.03	2.20 2.20 4.00)	Stiff to very stiff black sandy gravelly CLAY containing occasional smooth sub-rounded cobbles and boulders. Gravel is sub-angular fine to coarse		
.50-3.95 .50-3.95	CPT N=45 B5			4,7/9,12,11,13		da la la la la la la la		5-10-101-0	
				10/10/2006:DRY		(4.00)		0	
1.50-4.95 1.50-4.95	CPT N=50 B6			11/10/2006:DRY 4,8/12,13,14,11					A strategy and the provident of the
6.00-6.45 6.00-6.45	CPT N=40 B7			3,6/8,11,11,10	4.03		Very stiff brown slightly sandy gravelly CLAY containing smooth sub-rounded cobbles. Gravel is sub-angular fine to coarse		6 4 4 6 4 7 9 1 4 V 1 2 1 4 4 1 2 1 4 4 1 4 4 1 4 4 1 4 4 1 4 4 1 4 4 1 4 4 1 4 4 1 4 4 1 4 4 1 4 4 1 4 4 1 4 4
7.50-7.95 7.50-7.95	B8 CPT N=43			Water Strike(1) at 7.40m, rose to 7.00m in 20 mins. 4,7/9,10,11,13		(3.80)			1 · · ] o ] · · ] o ] · · ] o ] · · ] o
9.00-9.45 9.00-9.45	CPT N=38 B9			3,5/8,8,10,12					0 . 0 . 0 . 0 . 0
Pomortes					0.23	E 10.00			1
Remarks Standpipe in Chiselling fr	nstalled to 10.00m. rom 3.90m to 4.00m	for 0.25 h	ours. Wa	ter added from 0.50m	to 3.50m	Water addeo	d from 4.00m to 9.50m.	) Log By	9
							1:50	TR	/ト

		r Si	te Ir	vestigati	ons	Lto	k	Site Proposed	Develop	ment at F	ortmarno	ock, Co. D	)ublin	N	lumber BH02
Installatio WATER I	on Type MONITOR	ING	Dimensio Interna Diame	ons Il Diameter of Tube [A] = 50 ter of Filter Zone = 200 mm	mm			Client Ballymore Residential Ltd							ob lumber 06-693
			Location AS PL		Ground I	.evel (m ).23	OD)	Engineer J B Barry	& Partner	rs Ltd				s	Sheet 1/1
Water Vater	Instr (A)	Level (mOD)	Depth (m)	Description				G	roundwa	ter Strik	es Durin	g Drilling	)		
		10.03	0.20	Concrete			Depth Struck	Casing				Read	lings		Depti
				Cement/Bentonite Grout	Date	Time	(m)	Casing Depth (m)	Water	w Rate Strike	5 min	10 min	15 min	<b>20 min</b> 7.00	Depti Seale (m)
		9.23	1.00	Gravel Filter											
		8.73	1.50												
								Gr	oundwal	ter Obse	rvations	During [	Drilling		
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0					Date	Time	Depti Hole (m)	h Casing Depth (m)	Water Depth (m)	Water Level (mOD)	Time	Depth Hole (m)			Wate Leve (mOI
					10/10/06 11/10/06	0800	4.00	4.00	DRY		1700	4.00 10.00	4.00	DRY	
								Instr	ument G	roundwa	ter Obse	ervations			
<u></u>					Inst.	[A] Type	:		1						
				Slotted Standpipe		Ins	trumen	t [A]				Rem	arks		
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Boring Meth	nod	Casing	Diameter	stigatio	Ground	_		Client Ballymore Residential Ltd	BH Job Num 06-0	ber	
		Locatio	n PLAN		Dates 13	/10/200	06	Engineer J B Barry & Partners Ltd	Sheet 1/1		
Depth (m)	Sample / Tests	Casing Depth (m)	Water Depth (m)	Field Records	Level (mOD)	De (n (Thick	pth n) (ness)	Description	Legen	p. Water	
).00-0.50 ).50-0.95 ).50-0.95	B1 CPT N=20 B2			2,3/4,3,5,8	14.39	-	(0.40) 0.40	TOPSOIL Firm to stiff brown slightly sandy gravelly CLAY containing smooth sub-rounded cobbles. Gravel is sub-angular fine to coarse			
1.50-1.95 1.50-1.95	CPT N=19 B3			1,5/4,6,5,4		أعلماما	(1.70)				
2.50-2.95 2.50-2.95	CPT N=34 B4			2,4/6,8,10,10	12.69	ուսուհասեսու	2.10	Stiff to very stiff black sandy gravelly CLAY containing occasional smooth sub-rounded cobbles and boulders. Gravel is sub-angular fine to coarse	6156195195 6015015015		
3.50-3.95 3.50-3.95	CPT N=44 B5			3,7/10,9,11,14							
4.50-4.95 4.50-4.95	CPT N=47 B6			4,8/10,12,12,13		datatatatatatatatatatata					
6.00-6.45 6.00-6.45	CPT N=50 B7			4,6/9,12,12,17		delater determines	(7.90)				
7.50-7.95 7.50-7.95	CPT N=47 B8			3,7/9,10,12,16		ռեռուհունուն					
9.00-9.45 9.00-9.45	CPT N=44 B9			6,6/10,10,11,13		haranaharan					
Demarka				13/10/2006:DRY	- 4.79		10.00			राज्यत्र	
Remarks Standpipe i Chiselling fi	nstalled to 10.00m. rom 4.10m to 4.40m	for 0.5 ho	urs. Wate	er added from 0.50m	to 9.00m.			Scale (approx		gge	
								1:50 Figure	1	₹/Hŀ	

Installatio	n Type		Dimensio			Lto	k	Site Proposed Client	Developr	ment at P	ortmarnc	ock, Co. D	Jublin	4   	orehole lumber BH03
WATERI	MONITOR	ING	Diame	I Diameter of Tube [A] = 50 ter of Filter Zone = 200 mm	mm			Ballymore	Resident	tial Ltd					lumber 06-693
			Location AS PL		Ground Level (mOD)     Engineer       14.79     J B Barry & Partners Ltd										Sheet 1/1
Mater Mater	Instr (A)	Level (mOD)	Depth (m)	Description				G	roundwa	ter Strik	es Durin	g Drilling	1		
		14.59	0.20	Concrete	Data	Time	Depth	Casing	Inflow	v Rate		Read	lings		Depth Sealed (m)
				Cement/Bentonite Grout	Date	Time	Depth Struc (m)	k Casing k Depth (m)	innov	w Kale	5 min	10 min	15 min	20 min	(m)
0 		13.79	1.00	Gravel Filter											
<u><u></u></u>		13.29	1.50												
• • • • •								Gr	oundwat	er Obse	rvations	During D	) Drilling		
					-			Start of S	Shift			E	End of S	hift	
					Date	Time	Dept Hole (m)	h Casing Depth (m)	Water Depth (m)	Water Level (mOD)	Time	Depth Hole (m)	Casing Depth (m)	Water Depth (m)	Water Level (mOD
					13/10/06		(m)	(m)	(m)			10.00		DRY	
								Instr	ument G	roundwa	ter Obse	ervations			
					Inst.	[A] Type	:								
				Slotted Standpipe		Ins	trumer	nt [A]							
					Date	Time	Dept (m)	th Level (mOD)	1			Rem	arks		
0															
		4.79	10.00												
Remarks Upright c	cover fitted														

Boring Meth Cable Percu		2	Diamete Omm cas	r ed to 7.50m	Ground	<b>Level</b> 15.34		Client Ballymore Residential Ltd	Job Numb 06-69
		Locatio AS	n PLAN			/10/20		Engineer J B Barry & Partners Ltd	Sheet 1/1
Depth (m)	Sample / Tests	Casing Depth (m)	Water Depth (m)	Field Records	Level (mOD)	D (Thio	epth (m) :kness)	Description	Legend
0.00-0.50	В1					hilli	(0.40)	TOPSOIL	
).50-0.95 ).50-0.95	CPT N=19 B2			1,2/3,5,5,6	14.94	ليستشلمه	0.40	Firm to stiff brown slightly sandy gravelly CLAY containing smooth sub-rounded cobbles. Gravel is sub-angular fine to coarse	
1.50-1.95 1.50-1.95	CPT N=17 B3			2,3/4,3,5,5	13.34	فبأعلطط أعليكم	(1.60) 2.00	Stiff to very stiff black sandy gravelly CLAY containing occasional smooth sub-rounded cobbles and boulders.	
2.50-2.95 2.50-2.95	CPT N=50 B4			4,8/12,11,14,13		ատեսուն		occasional smooth sub-rounded cobbles and boulders. Gravel is sub-angular fine to coarse	0 10 10 0
3.50-3.95 3.50-3.95	CPT N=45 B5			3,7/9,12,11,13		databah databa			0 10 10 10
4.50-4.95 4.50-4.95	CPT N=50 B6			3,6/10,11,14,15		hhibbbbbbbbb			
6.00-6.30 6.00-6.45	CPT 50/150 B7			6,10/17,33		and and a databased and a state of the state	(8.00)		: 1911년 1 이 1910년 1910년 1910년 1910년 1910년 1911년 19
7.50-7.95	B8			11/10/2006:DRY		hhh			
7.50-7.95	CPT N=45			12/10/2006:6.20m 3,7/10,11,11,13		տեստեսե			
9.00-9.45 9.00-9.45	CPT N=47 B9			4,8/9,9,12,17		տուսեստես			
				12/10/2006:DRY	5.34	ليسي	10.00		
Remarks Standpipe j	nstalled to 10.00m. rom 3.80m to 4.00m			n 1940 a				Scale	) Logg

Glo	ovei	<sup>r</sup> Si	te Ir	vestigati	ons	Lto	k	Site Proposed	Develop	ment at P	ortmarno	ock, Co. D	ublin	N	Borehold Number BH04
nstallation WATER M	n Type //ONITOR	ING	Dimensio Interna Diame	ons I Diameter of Tube [A] = 50 ter of Filter Zone = 200 mm	mm			Client Ballymore	Residen	tial Ltd					Job Number 06-693
			Location AS PL/		Ground L	.evel (m 6.34	OD)	Engineer J B Barry	& Partne	rs Ltd				5	Sheet 1/1
Mater Mage	Instr (A)	Level (mOD)	Depth (m)	Description		1997 1997		G	roundwa	ater Strik	es Durin	g Drilling	ı		
		15.14	0.20	Concrete			Depth	Casing	5000	SI CIT		Read	ings		Dept
				Cement/Bentonite Grout	Date	Time	Depth Struck (m)	Casing Depth (m)	Inflo	w Rate	5 min	10 min	15 min	20 min	Dept Seale (m)
		14.34	1.00	Gravel Filter											
		13.84	1.50												
								Gr	oundwat	ter Obse	rvations	During D	rilling	I	<u> </u>
								Start of S	hift			i	End of SI	hift	
					Date	Time	Dept Hole (m)	h Casing Depth (m)	Water Depth (m)	Water Level (mOD)	Time	Depth Hole	Casing Depth (m)	Water Depth (m)	Wate Leve (mOl
000					11/10/06 12/10/06	0800	(m) 7.50	(m) 7.50	(m) 6.20	(mOD) 9.14	1700	(m) 7.50 10.00	7.50 10.00	DRY DRY DRY	
000															
								Instr	ument G	roundwa	ater Obse	ervations			
					Inst.	[A] Type	:								
				Slotted Standpipe		Ins	trumer	it [A]				Rem			
					Date	Time	Dept (m)	h Level (mOD)				Kem	arks		
010															
0															
		5.34	10.00												

Remarks Upright cover fitted.

Boring Meth Cable Percu			Diamete 0mm cas	r ed to 9.60m	Ground	Level 11.49	(mOD)	Client Ballymore Residential Ltd	Job Num 06-	iber 693
		Locatio AS	n FLAN		Dates 10	/10/20	06	Engineer J B Barry & Partners Ltd	Shee 1	et /1
Depth (m)	Sample / Tests	Casing Depth (m)	Water Depth (m)	Field Records	Level (mOD)	De ( (Thic	epth m) kness)	Description	Leger	d
0.00-0.50	B1						(0.40)	TOPSOIL		
).50-0.95 ).50-0.95	CPT N=23 B2			2,3/5,6,5,7	11.09	فأعلماما	0.40	Firm to stiff brown slightly sandy gravelly CLAY containing smaooth sub-rounded cobbles. Gravel is sub-angular fine to coarse	01-01-01-01-01-01-01-01-01-01-01-01-01-0	
1.50-1.95 1.50-1.95	CPT N=26 B3			2,4/4,6,8,8		մոննեննեն	(1.90)			
2.50-2.95 2.50-2.95	CPT N=38 B4			3,5/8,10,11,9	9.19	علىلىلىلىلىلىلىلىلىل	2.30	Stiff to very stiff black sandy gravelly CLAY containing occasional smooth sub-rounded cobbles and boulders. Gravel is sub-angular fine to coarse		
3.50-3.95 3.50-3.95	CPT N=42 B5			4,6/8,8,12,14		hhhh				
4.50-4.95 4.50-4.95	CPT N=51 B6			5,8/10,11,13,17		databah hatabah hataba				
6.00-6.45 6.00-6.45	CPT N=50 B7			4,9/11,12,13,14			(7.30)			
7.50-7.80 7.50-7.95	CPT 50/150 B8			7,11/17,33		and a state of the				
9.00-9.45 9.00-9.45	CPT N=50 B9			4,9/11,13,16,10		հանդես				
				10/10/2006:DRY	1.89	hhilit	9.60		<u> </u>	S.
Remarks Standpipe i Chiselling fi	nstalled to 9.60m. rom 7.75m to 7.90m	for 0.5 ho	urs. Chis	elling from 9.50m to 9	).60m for 1	hour.	Watera	added from 0.50m to 9.00m.	a) Log By	jge
								1:50 Figure	TR No.	/H

Installatio			Dimensi	nvestigati ons al Diameter of Tube [A] = 50 ter of Filter Zone = 200 mm			c	Client Ballymore		ment at P	orthame			L N	ob lumber 06-693
			Location AS PL		Ground I	<b>_evel (m</b> 1.49		Engineer J B Barry 8	& Partner	rs Ltd		1		S	iheet 1/1
egend <sup>Xater</sup>	Instr (A)	Level (mOD)	Depth (m)	Description				Gr	roundwa	ter Strik	es Durin	g Drilling	1		
	6	11.29	0.20	Concrete			Depth	Casing	1.0			Read	ings		Depth Sealed (m)
				Cement/Bentonite Grout	Date	Time	Depth Struck (m)	Casing Depth (m)	initov	v Rate	5 min	10 min	15 min	20 min	(m)
		10.49	1.00	Gravel Filter											
		9.99	1.50												
								Gro	oundwat	er Obse	rvations	During D	rilling		
					Date			Start of S				-	End of S	-	
					10/10/06	Time	Depth Hole (m)	Casing Depth (m)	Water Depth (m)	Water Level (mOD)	Time	Depth Hole (m) 9.60	Casing Depth (m)	Water Depth (m) DRY	Water Level (mOD
								Instru	ument G	roundwa	ter Obse	ervations			
					Inst.	[A] Type	:								
				Slotted Standpipe			trument	[A]					54		
					Date	Time	Depth (m)	Level (mOD)				Rem	arks		
0															
		1.89	9.60												

Depth (m) 0.00-0.50 0.50-0.95 0.50-0.95 1.50-1.95 1.50-1.95 2.50-2.95 2.50-2.95	Sample / Tests B1 CPT N=20 B2 CPT N=20 B3 CPT N=37 B4	Location AS Casing Depth (m)	n PLAN Water Depth (m)	Field Records 1,3/6,6,5,3 1,2/3,5,5,7	Dates 09 (mOD) 13.48	/10/2006 Depth (m) (Thickness) (0.40	Engineer J B Barry & Partners Ltd Description TOPSOIL Firm to stiff brown slightly sandy gravelly CLAY containing smooth sub-rounded cobbles. Gravel is sub-angular fine to coarse	Sheet 1/1 Legend
0.00-0.50 0.50-0.95 0.50-0.95 1.50-1.95 1.50-1.95 2.50-2.95	B1 CPT N=20 B2 CPT N=20 B3 CPT N=37	Casing Depth (m)	Water Depth (m)	1,3/6,6,5,3		(0.40) 	TOPSOIL Firm to stiff brown slightly sandy gravelly CLAY containing smooth sub-rounded cobbles. Gravel is sub-angular fine to	Legend
0.50-0.95 0.50-0.95 1.50-1.95 1.50-1.95 2.50-2.95	CPT N=20 B2 CPT N=20 B3 CPT N=37				13.48	0.40	Firm to stiff brown slightly sandy gravelly CLAY containing smooth sub-rounded cobbles. Gravel is sub-angular fine to	
2.50-2.95	B3 CPT N=37			1,2/3,5,5,7		(1.60)		<u></u>
			9		11.88	2.00	Stiff to very stiff black sandy gravelly CLAY containing occasional smooth sub-rounded cobbles and boulders.	
				4,6/7,9,10,11		ուսեստես	occasional smooth sub-rounded cobbles and boulders. Gravel is sub-angular fine to coarse	
3.50-3.95 3.50-3.95	CPT N=48 B5			5,8/11,12,13,12				
4.50-4.95 4.50-4.95	CPT N=48 B6			4,7/10,13,11,14				
6.00-6.45 6.00-6.45	CPT N=50 B7			3,6/9,11,13,17				
7.50-7.95 7.50-7.95	CPT N=50 B8			4,9/11,11,14,14				
9.00-9.45 9.00-9.45	CPT N=49 B9			5,6/10,12,13,14				101-101-101-101 101-101-101-101
Remarks				09/10/2006:DRY		10.00		
Standpipe inst	stalled to 10.00m. m 6.90m to 7.20m	for 0.5 ho	urs. Wate	er added from 0.50m	to 9.50m.		Scale (approx) 1:50	Logg By TR/H

Gle	ove	r Si	te In	vestigati	ons	Lto	k	Site Proposed I	Developr	ment at P	ortmarno	ock, Co. D	Jublin	N	Borehole Number BH06
Installati WATER	ion Type MONITOR	ING	Dimensio Interna Diamet	ns I Diameter of Tube [A] = 50 er of Filter Zone = 200 mm	mm			Client Ballymore	Residen	tial Ltd				1	lob lumber 06-693
			Location AS PL/	AN	Ground	Level (m 3.88	OD)	Engineer J B Barry 8	& Partner	rs Ltd				5	Sheet 1/1
egend	Instr (A)	Level (mOD)	Depth (m)	Description				Gr	oundwa	ter Strik	es Durin	g Drilling	1		
S S S S S S S S S S S S S S S S S S S		13.68	0.20	Concrete		_	Depth	Casing		Dete		Read	lings		Depth Seale (m)
				Cement/Bentonite Grout	Date	Time	Depth Struck (m)	Casing k Depth (m)	Innov	v Rate	5 min	10 min	15 min	20 min	(m)
		12.88	1.00	Gravel Filter											
		12.38	1.50												
								Gr	oundwat	ter Obse	rvations	During E	Drilling		
					31423124145			Start of S					End of S	1	1
					Date	Time	Dept Hole (m)	h Casing e Depth (m)	Water Depth (m)	Water Level (mOD)	Time	Depth Hole (m)	Casing Depth (m)	Water Depth (m)	Wate Leve (mOl
					09/10/06							10.00		DRY	
0 0 0 0								Instru	ument G	roundwa	ater Obse	ervations	3		
0					Inst.	[A] Type	:								
-0 -0				Slotted Standpipe		Ins	strumer	nt [A]							
01010					Date	Time	Dept (m)	th Level (mOD)				Rem	narks		
없는 것,		3.88	10.00												

Upright cover fitted.

# APPENDIX C

**Exploratory Trial Pit Logs** 

Glo	over Sit	e In		tigati	ONS			Site Proposed Development at Portmarnock Client	k, Co. Dublin	Trial Numl TP(	ber 01
	FRACKED					15.56		Ballymore Residential Ltd		Numl 06-6	
		Location AS	n PLAN		Dates 20	/09/2006	3	Engineer J B Barry & Partners Ltd		Shee 1/	
Depth (m)	Sample / Tests	Water Depth (m)	Field	d Records	Level (mOD)	Dept (m) (Thickn	th ) iess)	Description		Legen	d
0.50 1.00 2.00 2.50 Plan	D1 D2 D3 D4		20/09/2006	:DRY	15.31 15.16	00 ° ° ° ° ° ° ° ° ° ° ° ° ° ° ° ° ° °		TOPSOIL Firm light brown friable slightly sandy si CLAY. Sand is fine to coarse. Gravel i coarse Stiff grey brown friable slightly sandy slightly with occasional subangular cobbles. S Gravel is fine to coarse Stiff dark grey friable slightly sandy slightly with occasional subrounded to subanguis fine to coarse. Gravel is fine to coarse Complete at 2.50m	ightly gravelly CLAY and is fine to coarse. htly gravelly CLAY ular cobbles. Sand		
		¢.			•		· [	Pit stable and level on completion CBR test from surface using DCP equipr	ment		
	a		•	к я		K 1	*				
		٠	•	• •	•		·				
	· ·	*	•	* *	2003		•				
		÷		r a	8 <b>4</b> 3 - 3	48 S					
		2		8) (A	0.25	5 S	S	cale (approx) Logged B	y Figu	re No.	

Glo	ver Sit	e In	vest	igati	ons	Ltd	Site Proposed Development at Portmarnock, Co. Dublin	Trial Numi TP(	ber
TONNE TI	RACKED	Dimensi	ons			Level (mOD) 14.05	Client Ballymore Residential Ltd	Job Numl 06-6	
		Location AS	n PLAN		Dates 20	/09/2006	Engineer J B Barry & Partners Ltd	Shee 1/	1
Depth (m)	Sample / Tests	Water Depth (m)	Field	Records	Level (mOD)	Depth (m) (Thickness)	Description	Legen	d
0.50 1.00 2.00 3.00 Plan	D1 D2 D3 D4		20/09/2006:t	DRY	13.80 13.45 12.05 11.05		TOPSOIL         Firm brown friable slightly sandy slightly gravelly CLAY.         Sand is fine to coarse.         Gravel is subrounded to subangular fine to coarse.         Gravel is subrounded to subangular cobbles.         Stiff dark grey friable slightly sandy slightly gravelly CLAY with occasional subrounded to subangular cobbles.         Stiff dark grey friable slightly sandy slightly gravelly CLAY with occasional subrounded to subangular cobbles.         Sand is fore to coarse.         Gravel is coarse.         Gravel is subrounded to subangular cobbles.         Sand is fine to coarse.         Gravel is subrounded to subangular cobbles.         Sand is fine to coarse.         Gravel is subrounded to subangular fine to coarse         Complete at 3.00m		
							Pit stable and level on completion CBR test from surface using DCP equipment		
				*					
 	• •	•		8 <b>1</b>	a e				
* *		£	a o		2 5			re No.	

Glo		e In	vestigat		Ltd	Site Proposed Development at Portmarnock, Co. Dublin Client	Trial Pit Number TP03
TONNE TI	RACKED	Dimensi	ons	1000 ACC/2010	15.79	Ballymore Residential Ltd	Number 06-693
		Location AS	י PLAN	Dates 20	)/09/2006	Engineer J B Barry & Partners Ltd	Sheet 1/1
Depth (m)	Sample / Tests	Water Depth (m)	Field Records	Level (mOD)	Depth (m) (Thickness)	Description	Legend
0.50 0.70 1.00 2.00 2.00	D1 B1 D2 D4 D3		20/09/2006:DRY	15.54 15.19 13.89 13.19	(0.35) 0.60 (1.30) (1.30) (1.30)	TOPSOIL Firm brown friable slightly sandy slightly gravelly CLAY. Sand is fine to coarse. Gravel is subrounded to subangular fine to coarse Stiff grey brown friable slightly sandy slightly gravelly CLAY with occasional subangular cobbles. Sand is fine to coarse. Gravel is subangular fine to coarse Stiff dark grey friable slightly sandy slightly gravelly CLAY with occasional subrounded to subangular cobbles. Sand is fine to coarse. Gravel is subrounded to subangular fine to coarse Complete at 2.60m	J <u></u>
Plan .	× ×	0.65	× × 6	*	• •	Remarks Pit stabel and level on completion CBR test from surface using DCP equipment	
• •	• •	•		2	•••		
		٠		2 <b>1</b>			
		20		•			
	14 HAD	R		×			ure No. 6-693.TP03

	ver Sit	Dimensi		gati		Lta	Proposed Development at Portmarnock, Co. Dublin Client	Number TP04
	ked excavator	Dimensi	10113			8.48	Ballymore Residential Ltd	Number 06-693
		Location As	n Plan		Dates 20	/09/2006	Engineer J B Barry & Partners Ltd	Sheet 1/1
Depth (m)	Sample / Tests	Water Depth (m)	Field F	Records	Level (mOD)	Depth (m) (Thickness)	Description	Legend
0.50 1.00 2.00 2.50 Plan	D1 D2 D3 D4		20/09/2006:D	RY	8.28 7.98 7.88 5.88		TOPSOIL Firm light brown friable slightly sandy slightly gravelly LCAY Stiff brown grey friable slightly sandy slightly gravelly CLAY Complete at 2.60m Remarks	
i an ,		•	• •	٠	•	• •	Trial pit stable and dry on completion CBR test from surface using DCP equipment	
	in a		• •		e >	• •		
	• •							
		а Э	ст й 8 8	97.0 1970				
	tu tu						Scale (approx) Logged By Figur	e No.

Excavation	Method Ked excavator	Dimens		uya		round L	evel (mOD)	Proposed Development at Portmarnock, Co. Dublin Client Ballymore Residential Ltd	Job Numbo
		Locatio As	n Plan		D	ates	9/2006	Engineer J B Barry & Partners Ltd	06-69 Sheet 1/1
Depth (m)	Sample / Tests	Water Depth (m)	Fie	ld Record	is (i	_evel mOD)	Depth (m) Thickness)	Description	Legend
0.50 0.70 1.00 2.00 2.50	D1 B1 D2 D3 B2		20/09/2006	S:DRY		11.59	(0.35) 0.60 (1.20) 1.80 (0.80) 2.60	TOPSOIL Firm brown slightly friable slightly sandy slightly gravelly CLAY. Sand is fine to coarse. Gravel is subangular fine coarse Stiff grey brown friable slightly sandy slightly gravelly CL with occasional subangular fine to coarse Gravel is subangular fine to coarse. Gravel is subrounded to subangular fine to coarse Complete at 2.60m	AY arse.
Plan .	ас в	. <b>.</b> .	.*:	16 - C			•	Remarks trial pit stable and dry on completion CBR test from surface using DCP equipment	
	342 H	2		¥		¥			
• •			18 <b>7</b> ()	* 1		5			
							2		
							.	cale (approx) Logged By	Figure No.

	over Sit	1		igati				Site Proposed Development at	Portmarnock, Co. Dublin	Trial Pit Number TP06
tonne trac	Method ked excavator	Dimens	ions		Ground	Level 15.07	(mOD)	Client Ballymore Residential Ltd		Job Number 06-693
		Location As	n Plan		Dates 20	/09/20	006	Engineer J B Barry & Partners Ltd		Sheet 1/1
Depth (m)	Sample / Tests	Water Depth (m)	Field	I Records	Level (mOD)	D (Thic	epth m) kness)	D	escription	Legend
0.50 0.70 1.00 2.00 2.60	D1 B1 D2 B2 D3 D4		20/09/2006:	DRY	14.82 14.47 13.27 12.47		(0.25) 0.25 (0.35) 0.60 (1.20) 1.80 (0.80) 2.60	medium Stiff slightly friable grey bro gravelly CLAY with occasi is fine to coarse. Gravel is to coarse	htly sandy slightly gravelly rse. Gravel is subangular fine to own slightly sandy slightly onal subangular cobbles. Sand s subrounded to subangular fine dy slightly gravelly CLAY. Sand is ubrounded to subangular fine to	
Plan .		•	٠		•			Remarks trial pit stable and dry on co CBR test from surface using	mpletion a DCP equipment	
	380 B				۰ ۲	ĸ	•			
• •	ан н С		kar L		10 S					
			*) •)							
	· ·		÷		i i		. s	icale (approx)	Logged By Figu TR/KL 06	re No. -693.TP06

# APPENDIX D

**Results of Percolations Tests** 

Project No.		06-693 Proposed 1	Development	Portmarnock	Glover Site Investigations Ltd Full analysis not possible using method as			
Site		Proposed Development Portmarnock			described in BRE Digest 365/CIRIA Report			
Test Location							156	
test pit top dimensions		1	1.9	te	est infiltration rate (q) =	#DIV/0!	m/h	
test pit base dimensions				depth to groundwater before adding water = DRY				
test pit depth		1.3 m		depth to water surface at start of test 0.8				
						q	q from start of	
		depth of	time elapsed	volume of water lost from test start	Area of walls and base at 50% drop from test start	from start of test	test	
time (mins)	depth to water surface (m)	water in pit (m)	(mins)	(m3)	(m2)	(m/min)	(m/h)	
0	0.8	0.5	0	0	3.173076923	(	<u>, ;</u>	
	0.8	0.5	5	0	3.173076923	0.000	0.000	
5	0.8	0.5	10	0	3.173076923	0.000	0.000	
10	Actual for	0.5	15	0	3.173076923	0.000	0.000	
15	0.8	0.5	20	0	3.173076923	0.000	0.000	
20	0.8	0.5	60	0	3.173076923	0.000	0.000	
60	0.8	0.5	80	0	3.173076923	0.000	0.000	
80	0.8	0.5	115	0	3.173076923	0.000	0.000	
115	0.8	0.5		0	3.173076923	0.000	0.000	
160	0.8	0.5		0	3.173076923	0.000	0.000	
200	0.8	0.5		0	3.173076923	0.000	0.000	
240	0.8	0.5	240	0	5.175070725	0.000		
							-	
							1	
							2	
							2	
							2	
							5	
							2	
							21 21	
							#1 51	

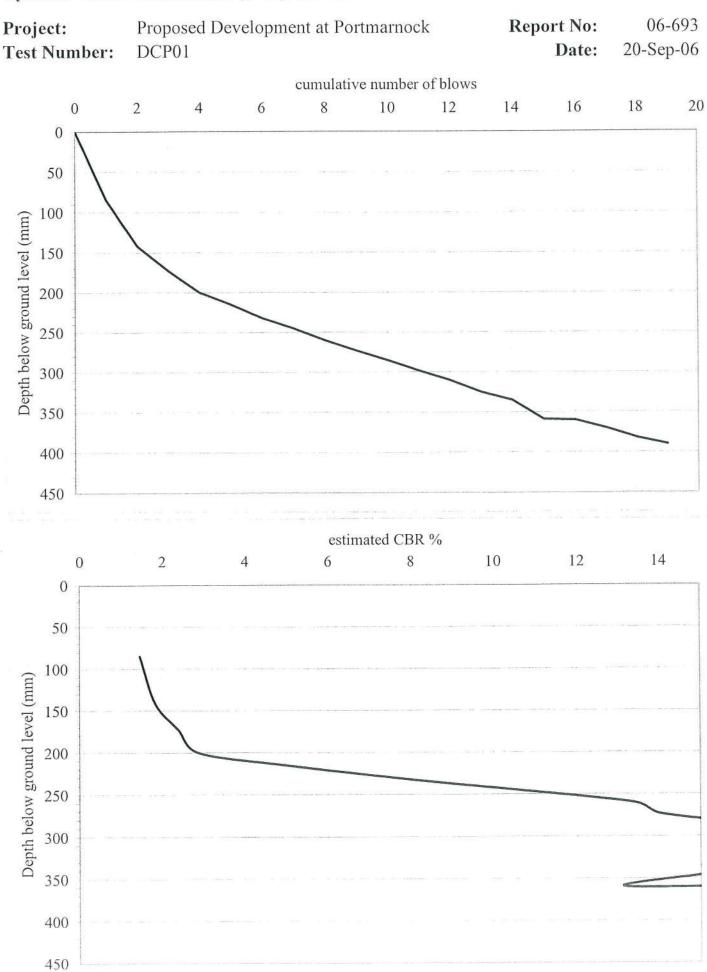
Project No.		06-693			Glover Site Investigations Ltd			
Site		Proposed Development Portmarnock			Full analysis not possible using method as			
Test Location		P2			described in BRE Digest 365/CIRIA Report			
		width (m)	length (m)				156	
test pit top dimensions		1	1.8	te	est infiltration rate (q) =	#DIV/0!	m/h	
test pit base dimensions		. 1	1.1	depth to gr	oundwater before addin	ig water =	DRY	
test pit depth		1.4 m		depth to water surface at start of test 0.75				
						q	q	
		depth of	time elapsed	volume of water	Area of walls and base at	from start	from start of	
	depth to water			lost from test start		of test	test	
time (mins)	surface (m)	(m)	(mins)	(m3)	(m2)	(m/min)	(m/h)	
0	0.75	0.65		0	4.04125			
5	0.75	0.65		0	4.04125	0.000	0.000	
10	0.75	0.65	10	0	4.04125	0.000	0.000	
15	0.75	0.65	15	0	4.04125	0.000	0.000	
20	0.75	0.65	20	0	4.04125	0.000	0.000	
60	0.75	0.65	60	0	4.04125	0.000	0.000	
80	0.75	0.65	80	0	4.04125	0.000	0.000	
115	0.75	0.65		0	4.04125	0.000	0.000	
160	0.75	0.65		0	4.04125	0.000	0.000	
200	0.75	0.65		0	4.04125	0.000	0.000	
240	0.75	0.65		0	4.04125	0.000	0.000	
240	0.75	0.05	240	0	1.01100	0.000	0.000	
						1	2	
							12 C	
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							1000 - 1000 - 1	
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							Ξ.	

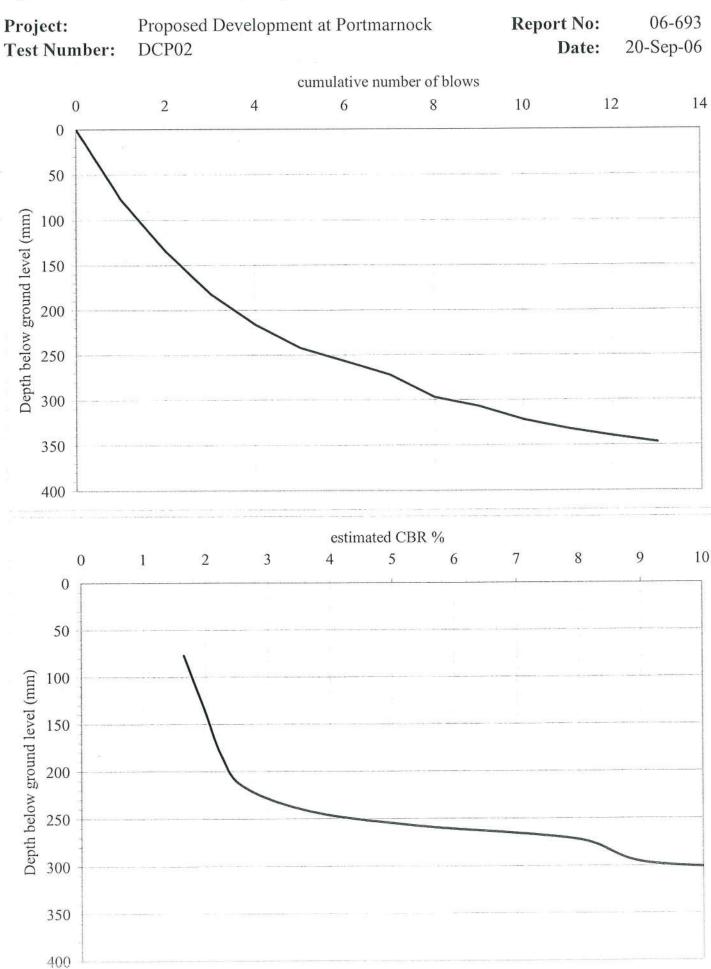
Project No.		06-693			Glover Site Investigations Ltd		
Site		Proposed Development Portmarnock			Full analysis not possible using method as		
Test Location		P3			described in BRE Digest 365/CIRIA Report		
		width (m) length (m)			156		
test pit top dimensions		1.1	2	2 test infiltration rate (q) =		#DIV/0! m/h	
test pit base dimensions		1	1.1	1.1 depth to groundwater before addin		13770 10	
test pit dep	test pit depth		m depth to water surface at start of te				
	depth to water	depth of water in pit	time elapsed from test start	volume of water lost from test start	Area of walls and base at 50% drop from test start	q from start of test	q from start of test
time (mins)	surface (m)	(m)	(mins)	(m3)	(m2)	(m/min)	(m/h)
0	0.8	0.5	0	0	3.392307692		
5	0.8	0.5	5	0	3.392307692	0.000	0.000
10	0.8	0.5	10	0	3.392307692	0.000	0.000
15	0.8	0.5	15	0	3.392307692	0.000	0.000
20	0.8	0.5	20	0	3.392307692	0.000	0.000
60	0.8	0.5	60	0	3.392307692	0.000	0.000
80	0.8	0.5	80	0	3.392307692	0.000	0.000
115	0.8	0.5	115	0	3.392307692	0.000	0.000
160	0.8	0.5	160	0	3.392307692	0.000	0.000
200	0.8	0.5	200	0	3.392307692	0.000	0.000
240	0.8	0.5	240	0	3.392307692	0.000	0.000

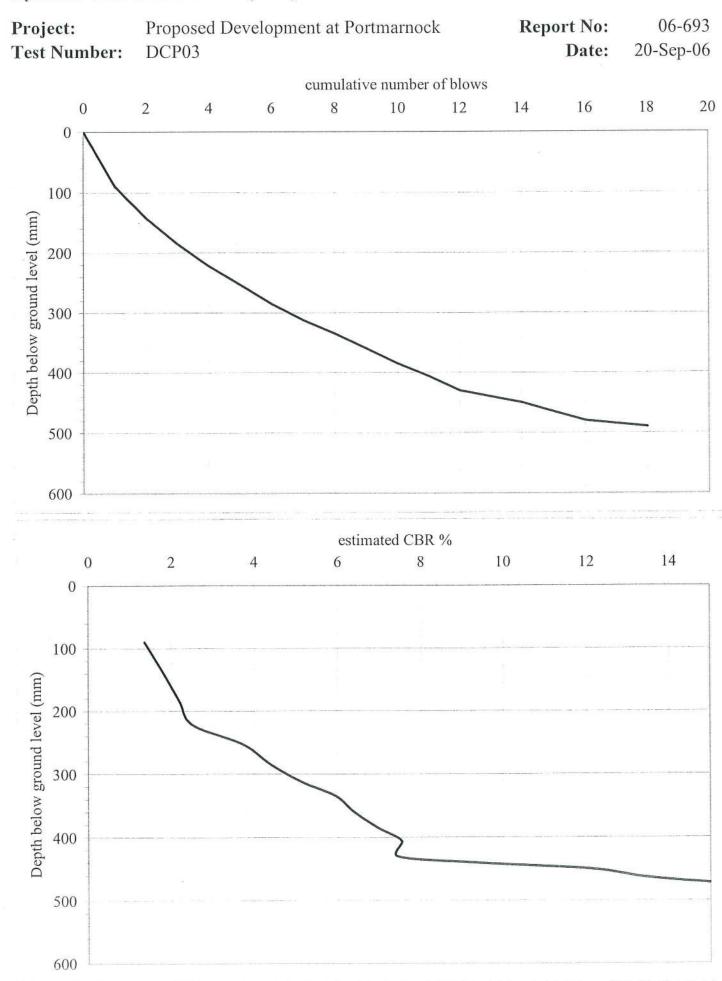
### APPENDIX E

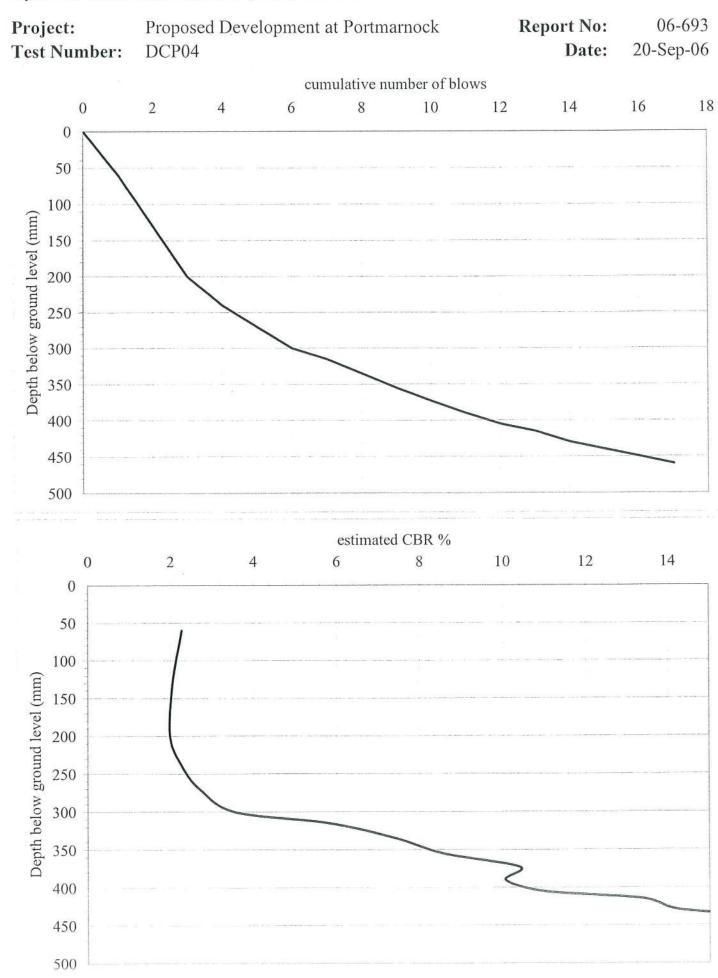
**CBR Test Results** 

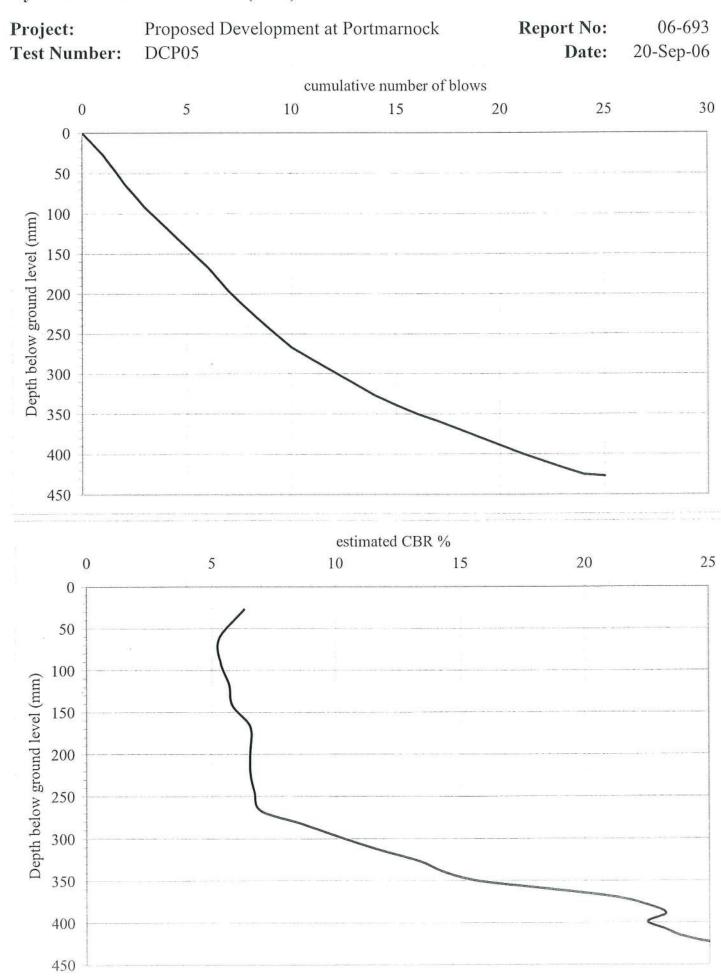
GLOVER SITE INVESTIGATIONS LIMITED

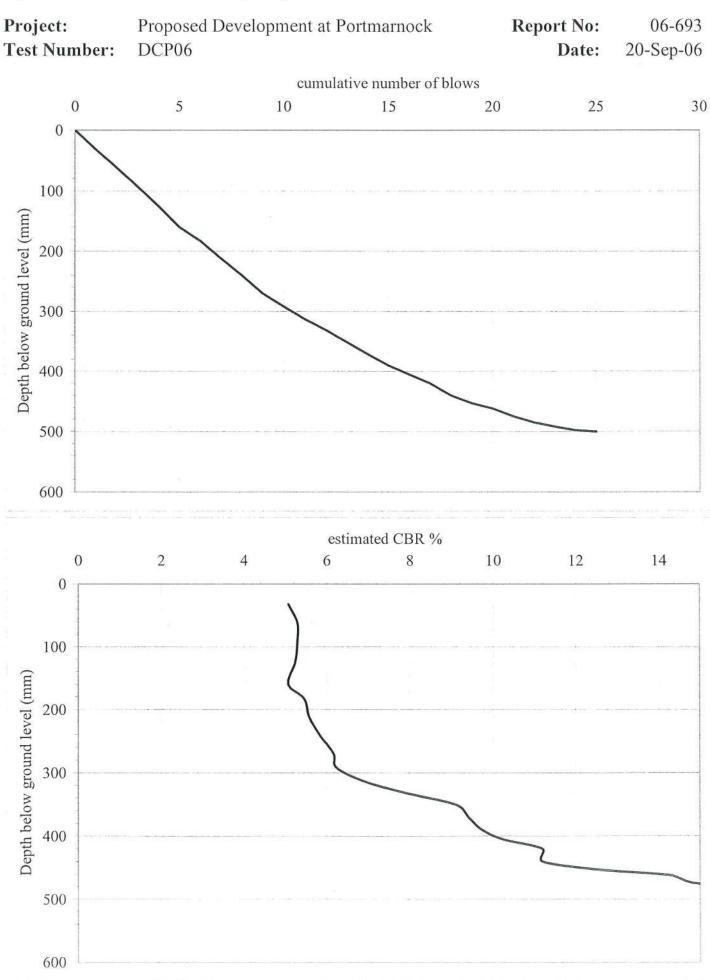












#### **APPENDIX F**

Laboratory Test Results

# i) pH and Water Soluble Sulphate Analysis

Contract: Proposed Development at Portmarnock, Co Dublin Job No.: 06-693

2.95

1.95

**Glover Site Investigations Ltd** 

42

66

Sheet 1 of 1

7

7.5

#### CHEMICAL TESTS Tests 3, 4, 5 & 9 of BS 1377 : Part 3 : 1990

4

2

B

B

2.50

1.50

EH

No.

**BH01** 

BH05

BH06

Sulphate Content SO4 pH Sample Passing Organic Mass Sample Depth Soil/water Groundwate Value Matter 2mm Loss on extract @ at Sieve Content Ignition r to Type No. (mg/l)(m) (m) (%) \* (%) # (%) \$ (mg/l) 8.0 2 0.50 0.95 24 В 7.5 1.50 60 **BH02** В 3 1.95 8.0 78 BH03 В 3 1.50 1.95 55 7.5 **BH04** B 2 0.50 0.95

\* Walkley and Black's dichromate method --Clause 3 # Average of 3 specimens - Clause 4 \$ Water soluble SO<sub>4</sub> from 2:1 water - soil extract - Clause 5.5 @ Average of 2/3 specimens - Clause 9.5 Clause Nos. of BS 1377 : Part 3 : 1990

## ii) Atterberg Limit Analysis and Moisture Content

Contract: Proposed Development at Portmarnock, Co.Dublin Job No.: 06-693

Sample

Depth

to (m)

at (m)

0.50

1.00

0.70

1.00

1.00

1.50

Moisture

Content

(%)

16

17

14

18

16

15

Passing

425mm

Sieve (%)

79

83

83

82

79

72

Liquid

Limit

(%)

36

34

33

35

37

34

Plastic

Limit

(%)

19

21

18

20

21

18

Plasticity

Index

(%)

17

13

15

15

16

16

Moisture content, Atterberg Limits & Particle Density Tests 3.2, 4.3, 5.3 & 8.3 of BS 1377 : Part 2 : 1990

Sample

No.

2

2

1

2

2

2

Туре

D

D

В

D

D

В

CL

Casagrande

Classification

CI

CL

CL

CL

CI

EH

No.

**TP01** 

**TP02** 

**TP03** 

**TP04** 

**TP05** 

**TP06** 

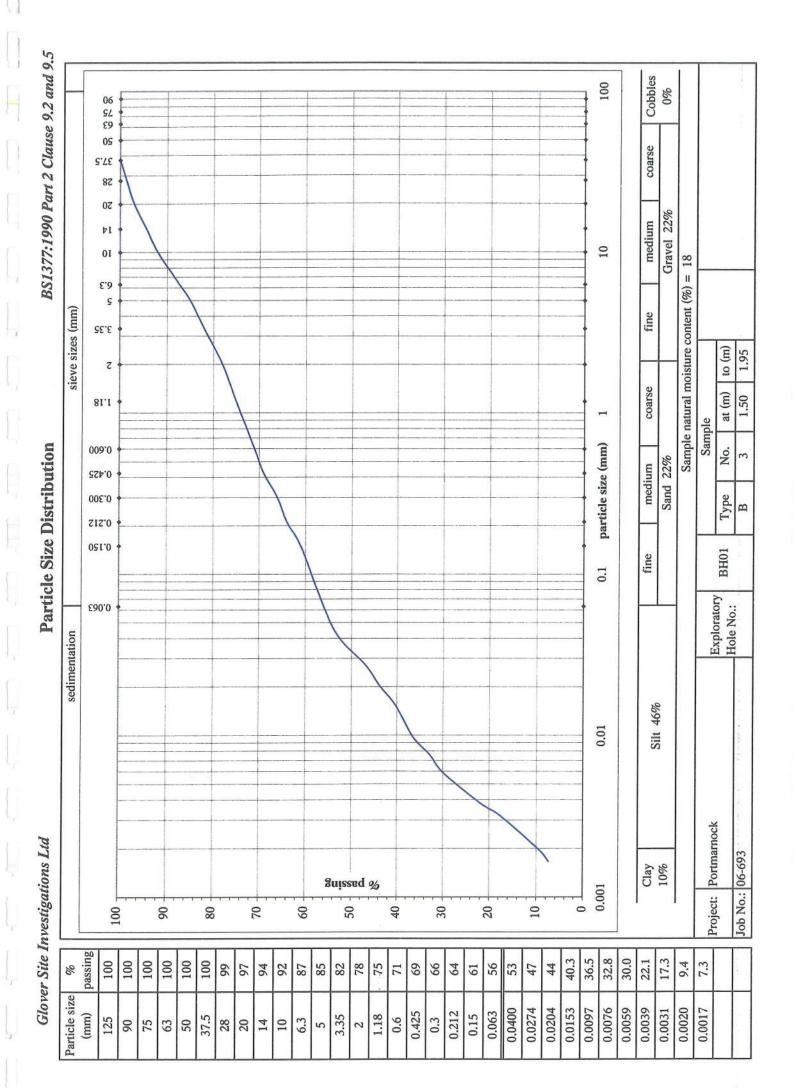
Sheet 1 of 1

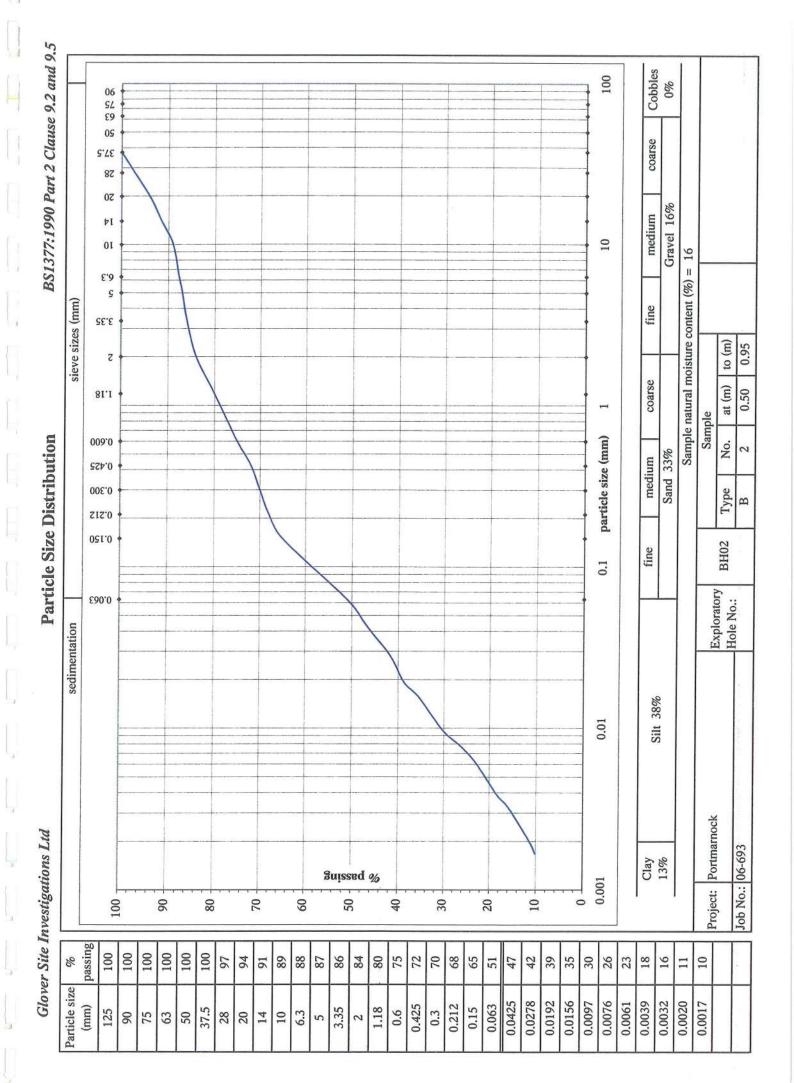
Particle

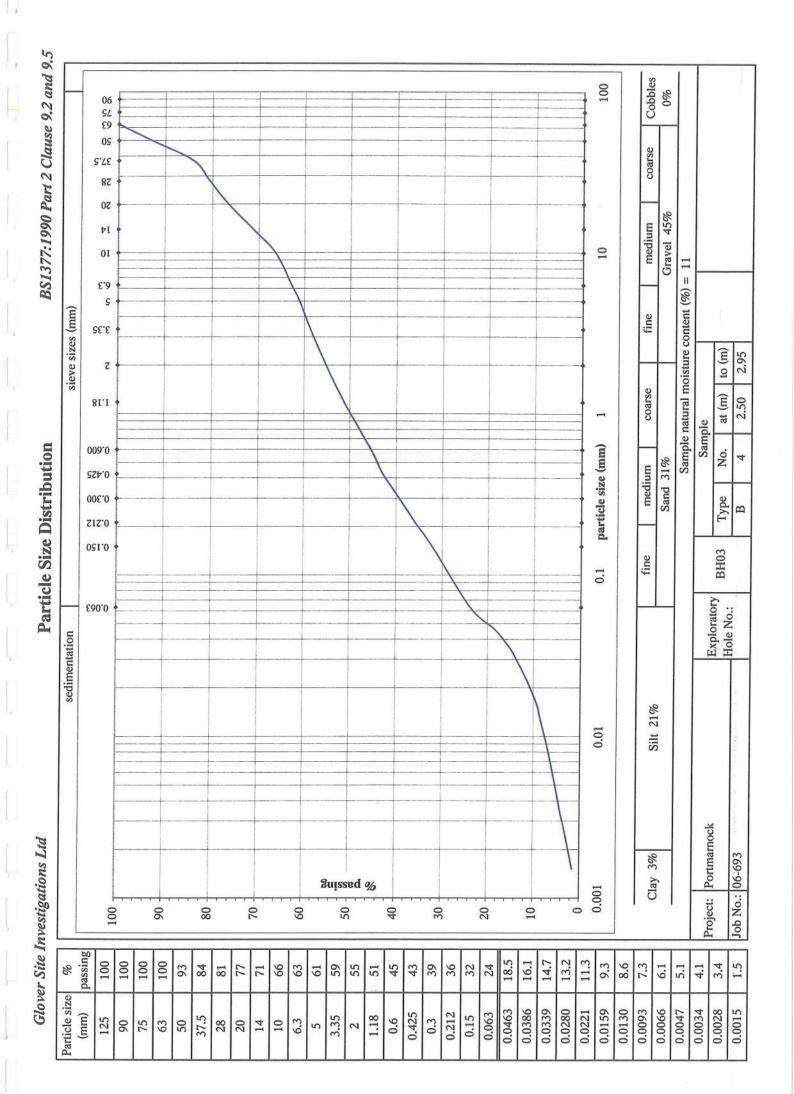
Density

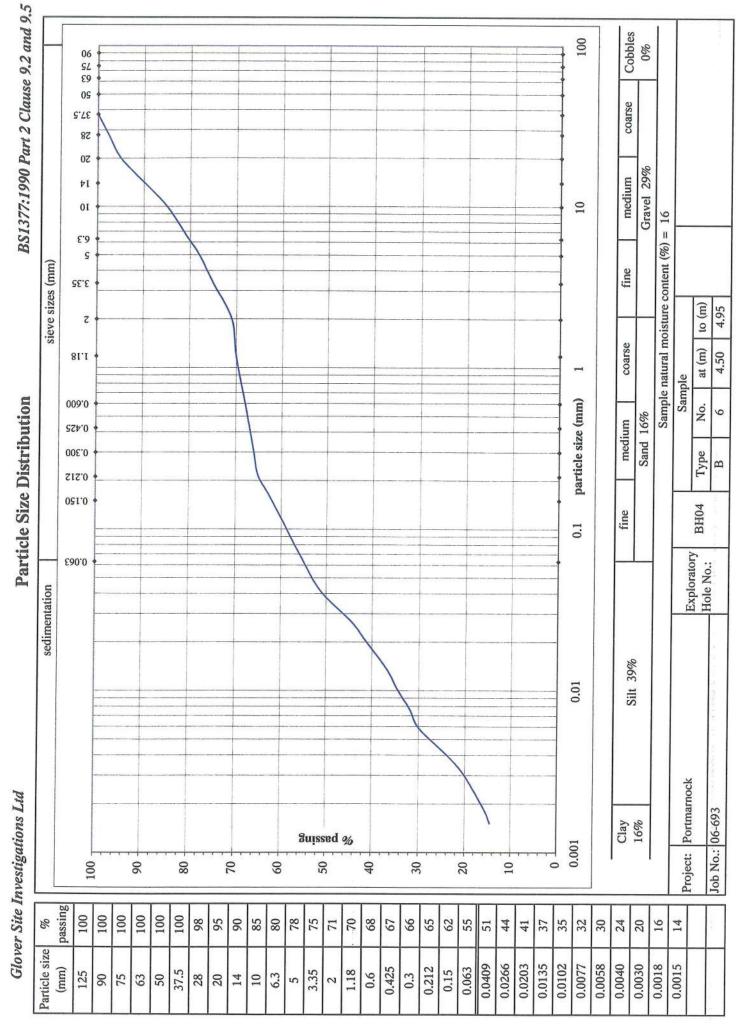
 $(Mg/m^3)$ 

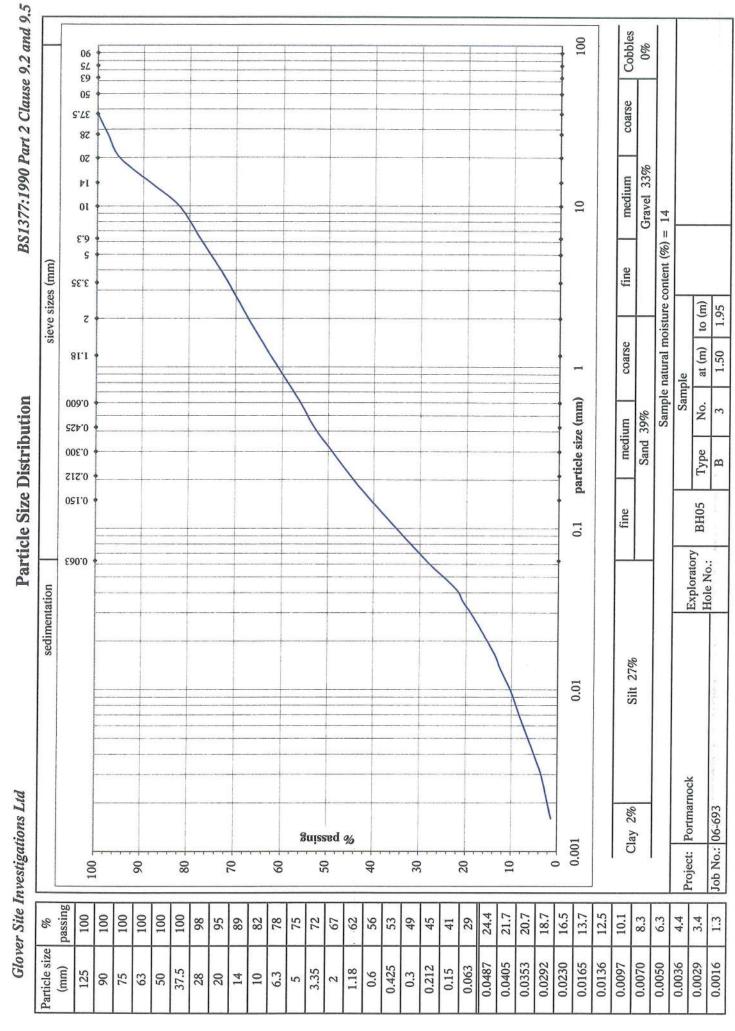
iii) Particle Size Distributions











#### APPENDIX G

Geology Maps of the Site

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